# Fragility Function Uncertainty Quantification in Infilled RC Frame Buildings

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## Outline





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**Statistical Significance in Regional Building Stocks** 



High Vulnerability to Ground-**Shaking Events** 



**Accurate Response** Characterisation



**Reduction of Uncertainty in Risk**and Loss-Based Applications



Improved Decision-Making and **Overall Community Resilience** 



Residential buildings by construction material



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**Statistical Significance in Regional Building Stocks** 



High Vulnerability to Ground-**Shaking Events** 



**Accurate Response** Characterisation





**Reduction of Uncertainty in Risk**and Loss-Based Applications





Improved Decision-Making and **Overall Community Resilience** 



Residential buildings by period of construction and construction material



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pre-1918

1919-1945

1946-1960

1961-1970 1971-1980 1981-1990 1991-2000 2001-2005 2006 and after





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Statistical Significance in Regional Building Stocks



High Vulnerability to Ground-Shaking Events



Accurate Response Characterisation





Improved Decision-Making and Overall Community Resilience













Damage observations following earthquake events



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Reduction of Uncertainty in Riskand Loss-Based Applications



Improved Decision-Making and Overall Community Resilience Predicted ≈ Observed Analytical >> Empirical



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High Vulnerability to Ground-Shaking Events



Accurate Response Characterisation





Reduction of Uncertainty in Riskand Loss-Based Applications

Improved Decision-Making and Overall Community Resilience

- Adequate characterization of structural response
- Reduction and mitigation of seismic risk
- Drafting prioritization schemes and policies
- Retrofitting and structural rehabilitation
- Adequate allocation of resources
- Minimization of direct and indirect seismic losses



## Simplified Assessment

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- Detailed numerical model;
- Must account for all possible inelastic mechanisms and failure modes;



### **Response Evaluation Tool**

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- Perform eigenvalue analysis to extract first-mode shape ordinates;
- Perform nonlinear static pushover to characterize the lateral response of the MDOF system (i.e., base-shear vs roof displacement);



#### **Response Evaluation Tool**

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Equivalent SDOF Definition



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Total 105 low-rise and mid-rise infilled RC building

archetypes



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spectral acceleration corresponding to conditioning

*periods of T*\*=0.2-0.6s



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#### Suggested value of IM EDP uncertainty for low and mid-rise infilled RC

buildings

Seismic Code Level	Number of Stories	Taxonomy Code	Suggested Dispersion, β
Low (GLD)	Low-rise (2-3)	LC-LR	0.25
	Mid-rise (4-6)	LC-MR	0.23
Moderate (SSD)	Low-rise (2-3)	MC-LR	0.28
	Mid-rise (4-6)	MC-MR	0.23



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# Case Study Example

- Three-storey RC school building with masonry infills;
- Located in Napoli, Italy;
- Constructed in the 1960s, before the introduction of modern seismic design guidelines;



General layout and numerical modelling techniques of the case study school building.



#### Summary of the modal properties of the case study building in both principal directions

	X-Direction		Y-Direction (Weaker Direction)				
Floor No.	Mass, m <sub>i</sub> [tonnes]	First-mode shape, $\Phi$	Period, T <sub>1</sub> [s]	Yield spectral acceleration, Sa <sub>y</sub> [g]	First-mode shape, $\Phi$	Period, T <sub>1</sub> [s]	Yield spectral acceleration, Sa <sub>y</sub> [g]
Base	0	0.00	0.62	0.62 0.42	0.00	0.36	0.40
First	985	0.22			0.22		
Second	960	0.56			0.57		
Third	806	1.00			1.00		









#### Summary of the fragility function comparisons

Ductility Thresholds	Simplified Assessment		Extensive Assessment (MSA)	
	Median intensity, Sa <sub>avg</sub> [g]	Dispersion, β	Median intensity, Sa <sub>avg</sub> [g]	Dispersion, β
µ=1	0.20		0.22	0.28
μ=2	0.24	0.25	0.27	0.26
μ=5	0.44		0.43	0.26



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#### Summary of the fragility function comparisons



# Case Study Example

Seismic Risk Calculation:

• Classical Approach:

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$$\lambda = \int_0^{+\infty} P[\mu \ge \mu | IM = s] | dH(s) |$$

- Pushover-Based Risk Estimation (PB-Risk):
- 1. Second-order approximation of the hazard function:

$$H(s) = k_0 \exp\left[-k_2 ln^2(s) - k_1 \ln(s)\right]$$

2. Application of IM-based closed form expressions:

$$\lambda = \sqrt{pk_0^{1-p}} [H(s)]^p \exp\left[\frac{k_1^2}{4k_2}(1-p)\right]$$
$$p = \frac{1}{1+2k_2\beta^2}$$

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Seismic hazard characterization and second-order approximation



#### Summary of the risk assessment comparisons

Ductility Thresholds	PB-Risk	Classical	Error in MAEE antimation	
THESHOLDS	Mean annual frequency of exceedance (MAFE), $\lambda$		Error in MAPE estimation	
μ=1	0.0031	0.0029	6.89%	
μ=2	0.0021	0.0019	10.52%	
μ=5	4.92E-04	5.02E-04	1.6%	



### Conclusion

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- A simplified tool for the seismic performance assessment of infilled RC frames was presented;
- The epistemic uncertainty associated with structural response (IM|EDP) was quantified for the infilled RC typology and different sub-taxonomies;
- The suggested values of the record-to-record variability could be incorporated with other simplified methodologies for the derivation of fragility functions or risk metrics (e.g. *PB-Risk*);
- The suggested values were validated on a case-study school buildings where an adequate match in terms of vulnerability and risk parameters was observed;
- Good agreement between the results of classical and simplified methodologies;



#### Links:

• Database of Archetype Building Models:

https://github.com/gerardjoreilly/Infilled-RC-Building-Database

• Response Estimation Tool for Infilled RC Frame Structures:

https://github.com/gerardjoreilly/Infilled-RC-Building-Response-Estimation

#### Publications:

- Nafeh A.M.B., O'Reilly G.J. (2022) Unbiased simplified seismic fragility estimation of non-ductile infilled RC structures. Soil Dynamics and Earthquake Engineering 157:107253. <u>https://doi.org/10.1016/j.soildyn.2022.107253</u>
- <u>PB-Risk</u>: Nafeh, A.M.B., O'Reilly, G.J. (2023) Simplified pushover-based seismic risk assessment methodology for existing infilled frame structures. Bulletin of Earthquake Engineering 21, 2337– 2368 <u>https://doi.org/10.1007/s10518-022-01600-y</u>



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